1. COORDINATION:

CONTRACTOR TO COORDINATE ALL PIPE AND CONDUIT LOCATIONS WITH MECHANICAL AND ELECTRICAL DRAWINGS PRIOR TO PLACING CONCRETE. CONTRACTOR SHALL PROVIDE SHOP DRAWINGS FOR EQUIPMENT AND ANCHOR BOLT LOCATIONS.

STRUCTURAL DRAWINGS SHALL BE COORDINATED WITH MECHANICAL AND ELECTRICAL DRAWINGS TO PROPERLY LOCATE WALL PIPES, PIPE SLEEVES, ANCHOR BOLTS, BLOCKOUTS, ETC. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE COUNTY ENGINEER BEFORE PROCEEDING WITH THE WORK.

2. PRECAUTIONS:

CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO PREVENT FLOTATION OF STRUCTURES UNTIL FULLY CONSTRUCTED AND BACKFILL IS IN PLACE AND COMPACTED.

3. DESIGN CRITERIA AND LOADS:

ACI 350 CONCRETE SANITARY ENGINEERING STRUCTURES ACI 318 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE

ASTM C 478 STANDARD SPECIFICATION FOR CIRCULAR PRECAST REINFORCED CONCRETE MANHOLE SECTIONS ASTM C 433 STANDARD SPECIFICATION FOR JOINTS FOR CONCRETE

PIPE AND MANHOLES, USING RUBBER GASKETS

DESIGN LIVE LOADS:

AASHTO HS20-44 WET WELL TOP SLAB AASHTO HS20-44. SITE PAD - VEHICULAR AREA BEARING

SITE PAD - NON-VEHICULAR AREA BEARING 300 PSF.

NET ALLOWABLE SOIL BEARING CAPACITY: DESIGN ENGINEER TO CONFIRM

4. CAST-IN-PLACE CONCRETE:

CAST-IN-PLACE CONCRETE SHALL HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS:

SLABS ON GRADE 4.000 PSI PIPE SUPPORTS, PUMPS PADS, ENCASEMENTS 4,000 PSI

5. PRECAST CONCRETE:

PRECAST WET WELL CONCRETE SHALL HAVE MINIMUM 4.000 PSI COMPRESSIVE STRENGTH AT 28 DAYS.

6. REINFORCING STEEL:

REINFORCING STEEL FOR ALL BARS SHALL CONFORM TO ASTM 615. GRADE 60 OF UNITED STATES MANUFACTURE.

WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.

7. REINFORCEMENT CLEARANCE:

CLEARANCE OF REINFORCING STEEL FROM THE FACE OF CONCRETE TO THE OUTERMOST TIE OR BAR SHALL BE 2", UNLESS OTHERWISE NOTED ON THE DRAWINGS.

8. PIPE OPENINGS:

OPENINGS SHALL BE LOCATED BASED ON THE REQUIREMENTS SPECIFIED IN THE MECHANICAL DRAWINGS. MANUFACTURER RESPONSIBLE FOR COORDINATING SIZE AND LOCATION WITH ENGINEER.

9. ALUMINUM ACCESS HATCH:

ACCESS HATCH COVER SHALL BE ALUMINUM. MEET HILLSBOROUGH COUNTY SPECIFICATIONS AND BE SIZED AND DETAILED TO MEET THE REQUIREMENTS OF THE SELECTED PUMPS.

10. WATERSTOPS:

WATERSTOPS SHALL BE DUMBELL STYLE. MADE OF FLEXIBLE PVC AND MEASURING 6" x 3/8".

11. COATINGS:

THE WET WELL INTERIOR SHALL RECEIVE A FIELD APPLIED CORROSION RESISTANT COATING AS PER HILLSBOROUGH COUNTY SPECIFICATIONS.

12. SHOP DRAWINGS:

THE FOLLOWING SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW. FABRICATION SHALL NOT COMMENCE UNTIL ALL REVIEWS ARE COMPLETED.

- REINFORCING STEEL - ANCHOR BOLTS FOR PUMPS - PRECAST CONCRETE - CONCRETE MIX DESIGNS - GROUTS - ACCESS HATCHS AND FRAMES - WATERPROOF JOINTS - EXPANSION JOINT MATERIAL - LINKSEAL STYLE CONNECTION - FLEXIBLE 'BOOT' CONNECTIONS

13. FOUNDATIONS:

- GASKETS

REMOVE ALL ORGANIC SOIL. CLAYS AND OTHER COMPRESSIBLE MATERIALS.

A MINIMUM OF 6 INCHES OF COMPACTED CRUSHED STONE (SEE HILLSBOROUGH COUNTY STANDARDS) SHALL BE PLACED UNDER THE WET WELL BASE SLAB AND DROP CONNECTION.

- WATERSTOPS

DEWATER EXCAVATION DURING WET WELL INSTALLATION. ALL WORK SHALL BE DONE IN THE "DRY".

14. DESIGN CONDITIONS VS SITE CONDITIONS:

THE WET WELL DESIGN WALL THICKNESS, BOTTOM SLAB THICKNESS AND DIMENSIONS, BOTTOM SLAB EXTENSION, AND FOUNDATION BASE COURSE ARE MINIMUM DIMENSIONS. THE STRUCTURAL DESIGN IS BASED ON THE LOADS AND CONDITIONS LISTED HERE. TO USE THESE PLANS AS-IS, THE ENGINEER SHALL VERIFY THAT THE SITE CONDITIONS MEET THE DESIGN CONDITIONS, INCLUDING THE GEOTECHNICAL CONDITIONS AND FLOTATION CALCULATIONS. IF THE SITE CONDITIONS VARY FROM THE DESIGN CONDITIONS, THE ENGINEER SHALL MODIFY THE DESIGN AS NEEDED AND PROVIDE SIGNED AND SEALED DRAWINGS TO THE COUNTY FOR APPROVAL.

15. WET WELL CONSTRUCTION NOTES:

- A. GRAVITY PIPE, DISCHARGE PIPING, AUXILIARY SUCTION AND PROCESS PIPING SIZES TO BE DETERMINED BY THE ENGINEER. REFER TO MECHANICAL DRAWINGS.
- B. OPENING REQUIREMENTS:

GRAVITY PIPE: CAST-IN-PLACE OR CORED OPENING

FOR FLEXIBLE 'BOOT' CONNECTION.

DISCHARGE PIPING: LINK SEAL TYPE PIPE SEAL AUXILIARY PIPING: LINK SEAL TYPE PIPE SEAL NON-SHRINK GROUT. OTHER PIPING:

- C. DROP CONNECTION SHALL BE USED WHEN THE ELEVATION DIFFERENCE BETWEEN THE INVERT OF THE INFLUENT PIPE AND THE PUMP LOW WATER LEVEL IS GREATER THAN 2 FEET.
- D. WET WELL BARREL JOINTS SHALL BE SEALED WITH A SINGLE OFFSET OR O-RING STYLE GASKET PER ASTM C433.
- E. TOP SLAB. BOTTOM SLAB AND WET WELL BARREL SHALL BE PRECAST.

USE OF DRAWINGS:

SUCCESSOR PROFESSIONAL ENGINEER UTILIZING THESE DRAWINGS MUST ABIDE BY THE RULES AND REGULATIONS CONTAINED IN 61G15-27.001 OF THE FLORIDA ADMINISTRATIVE CODE.

> DAVID A. WILCOX FLORIDA PE 34942 PROJECT MANAGER

WILLIAM N. HAUSHEER FLORIDA PE 31715 STRUCTURAL ENGINEER

SCALE REVISIONS AS SHOWN No. DATE DESCRIPTION APPV'D.



1500 PSF. (MINIMUM)

HILLSBOROUGH COUNTY PUBLIC UTILITIES DEPARTMENT mana haman 925 E. TWIGGS STREET / TAMPA, FLORIDA 33602

	PROJECT No.:	
	FILE No.:	
	DESIGNED BY:	RMA
	DRAWN BY:	TRS
	CHECKED BY:	DAW & WNH
2	DATE:	SEPTEMBER 2015
	SCALE:	AS SHOWN

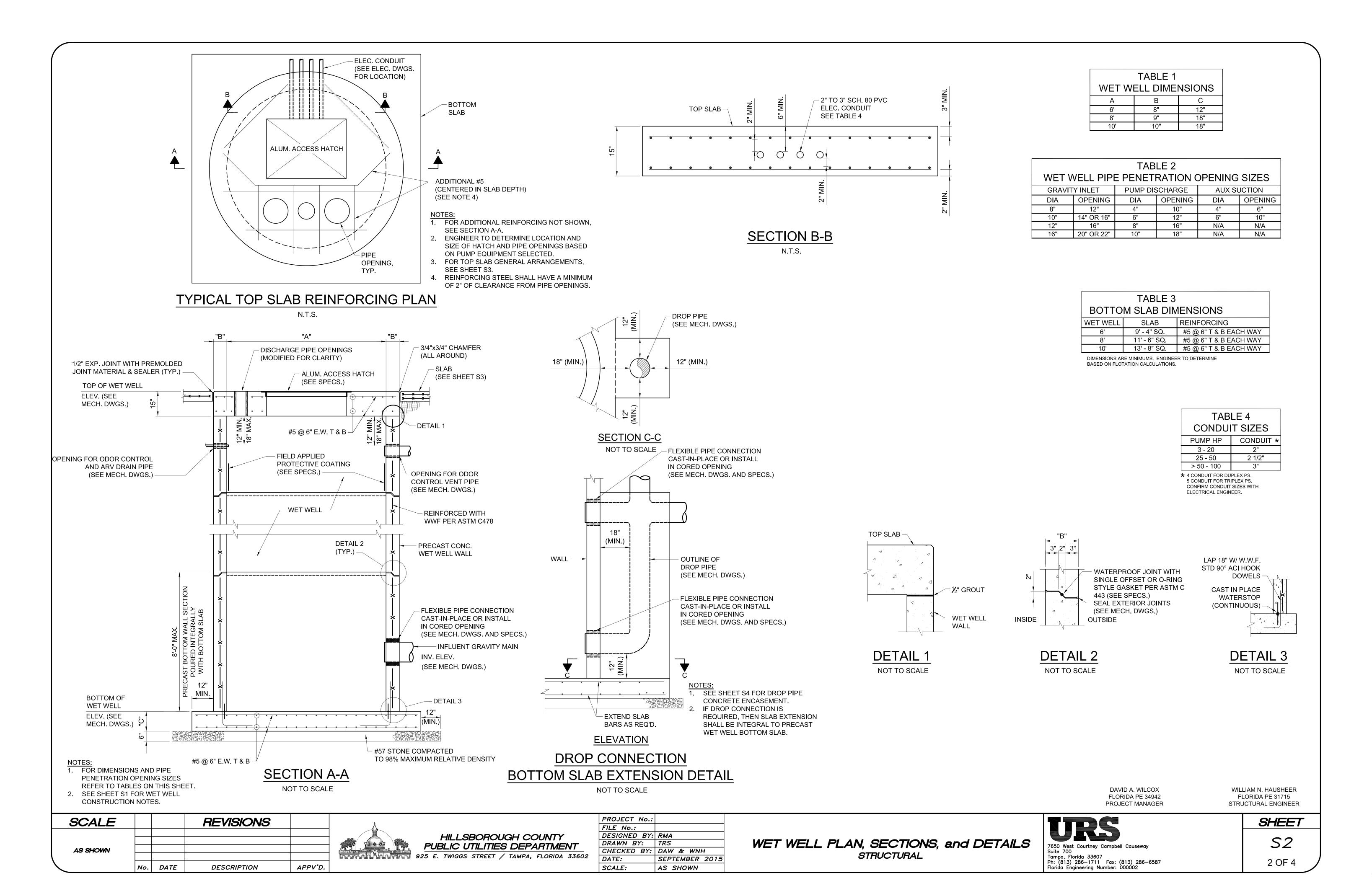
NOTES STRUCTURAL



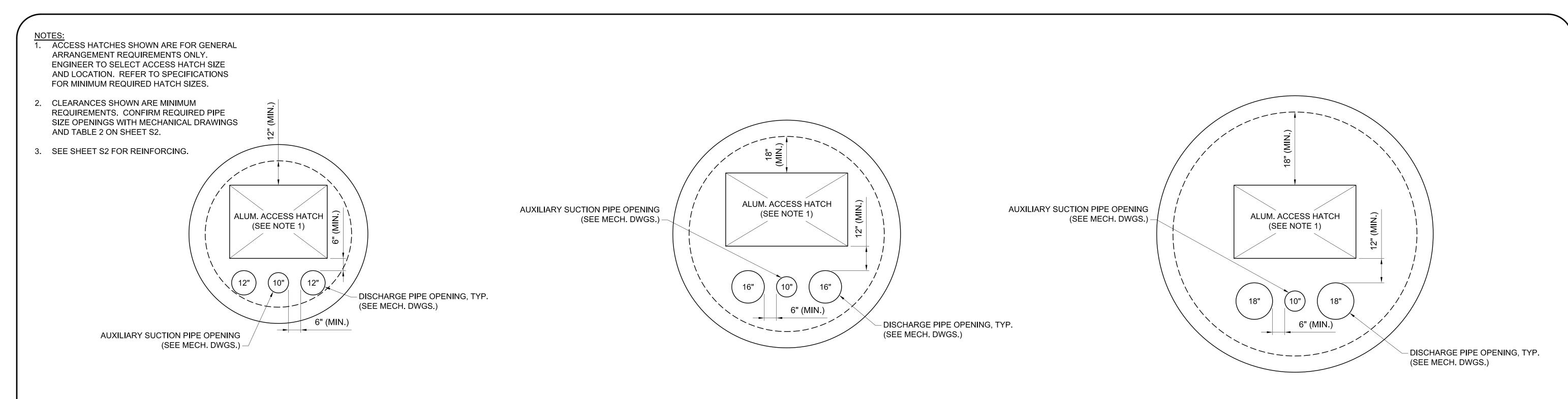
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SHEET

1 OF 4



(i) I



6' DUPLEX WET WELL TOP PLAN

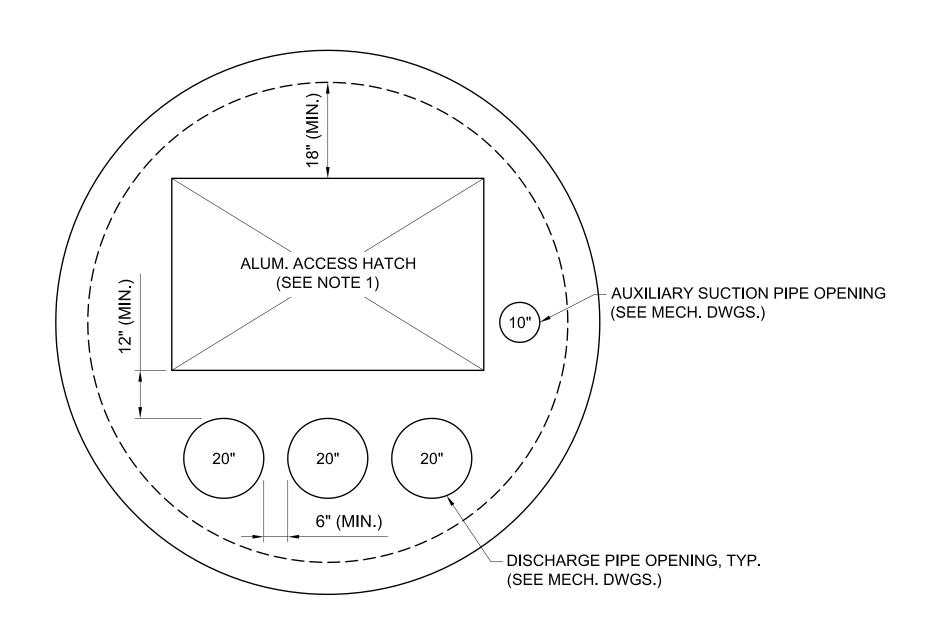
SCALE: 1/2" = 1'-0"

8' DUPLEX WET WELL TOP PLAN

SCALE: 1/2" = 1'-0"

10' DUPLEX WET WELL TOP PLAN

SCALE: 1/2" = 1'-0"



10' TRIPLEX WET WELL TOP PLAN

SCALE: 1/2" = 1'-0"

DAVID A. WILCOX FLORIDA PE 34942 PROJECT MANAGER

SCALE			REVISIONS	
AS SHOWN				
A3				
	No.	DATE	DESCRIPTION	APPV'D.

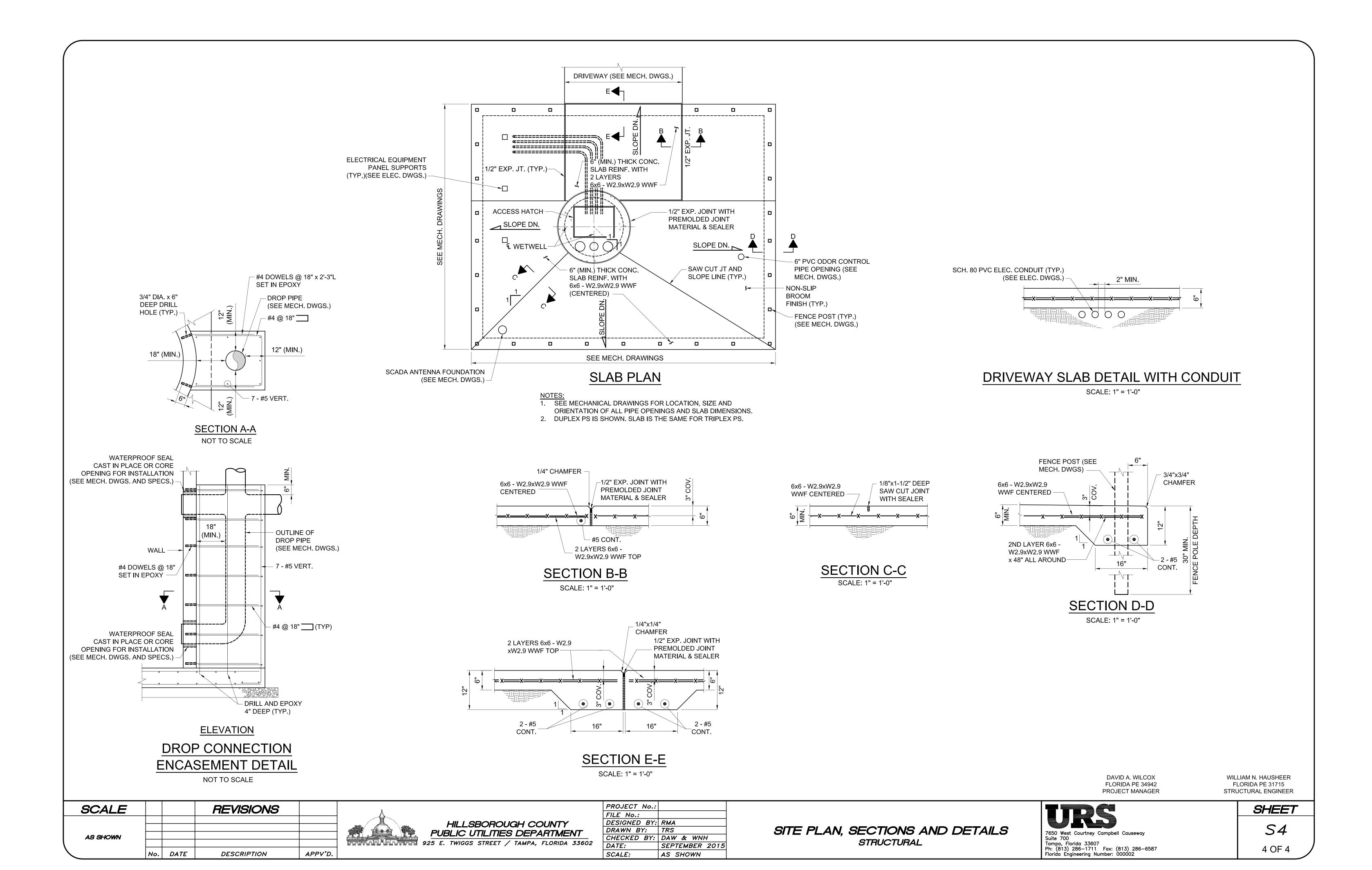
HILLSBOROUGH COUNTY
PUBLIC UTILITIES DEPARTMENT
925 E. TWIGGS STREET / TAMPA, FLORIDA 33602

	PROJECT No.:		
	FILE No.:		
	DESIGNED BY:	RMA	
	DRAWN BY:	TRS	
_	CHECKED BY:	DAW & WNH	•
2	DATE:	SEPTEMBER 2015	
	SCALE:	AS SHOWN	

WET WELL TOP SLAB GENERAL ARRANGEMENT STRUCTURAL

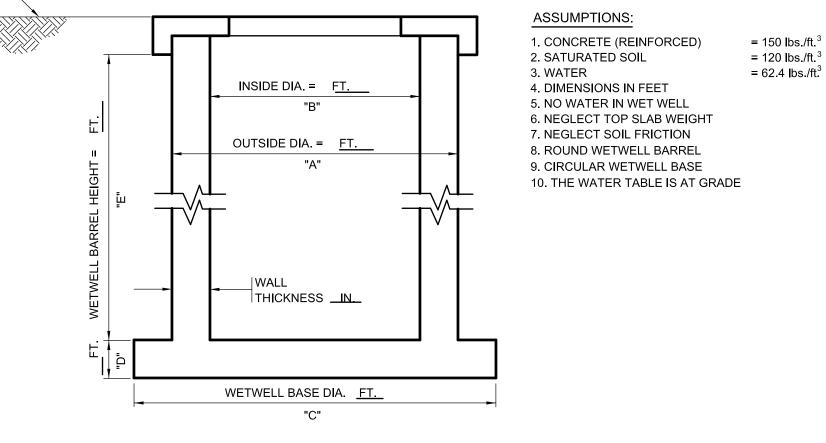
,_	URS
"	7650 West Courtney Campbell Causeway Suite 700 Tampa, Florida 33607 Ph: (813) 286-1711 Fax: (813) 286-6587 Florida Engineering Number: 000002

SHEET S3 OF 4



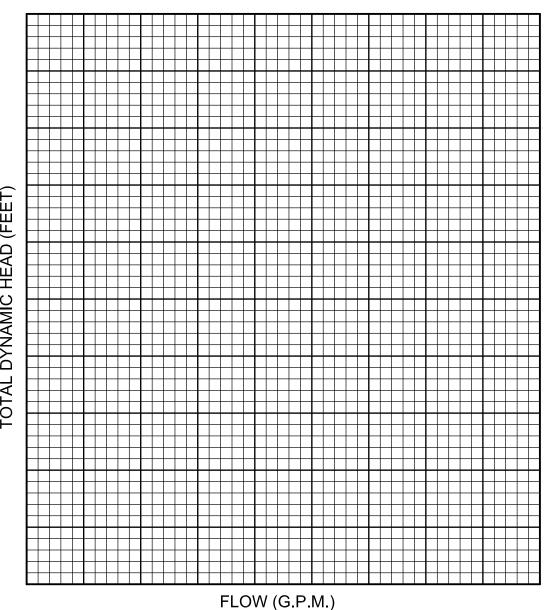
GRADE WASTEWATER PUMPING STATION DESIGN PARAMETERS (FOR COUNTY-OWNED STATIONS) INSIDE DIA. = FT. WASTEWATER PUMPING STATION: _____ OUTSIDE DIA. = FT. LOCATION: _____ SECTION: ____ TOWNSHIP: ____ RANGE: ____ I. SERVICE AREA □ NORTHWEST NAME OF WWTP SERVING THIS DEVELOPMENT: □ CENTRAL □ SOUTH II. DESIGN CAPACITY A. AVERAGE DAILY FLOW (A.D.F.): _____ x 200 GPD/UNIT (single-family units) _____ x 140 GPD/UNIT (multi-family units) "C" (1) BARREL WEIGHT = $(A^2 - B^2)(\pi / 4)(E)(150) = [(___)^2 - (___)^2](\pi / 4)(__)(150) =$ B. PEAK INFLUENT RATE (PEAK FACTOR = PER UTILITY TECHNICAL MANUAL, APPENDIX 5) (2) BOTTOM SLAB WEIGHT = $(C)^2 (\pi /4)(D)(150) = (\underline{})^2 (\pi /4)(\underline{})(150) =$ PEAK FACTOR = (PER UTILITY TECHNICAL MANUAL, APPENDIX 5) (3) SOIL WEIGHT = $(C^2 - A^2)(\pi/4)(E)(120-62.4) = [(___)^2 - (__)^2](\pi/4)(__)(57.6) =$ (4) TOTAL WEIGHT = (Barrel Weight) + (Bottom Slab Weight) + (Soil Weight) = (____) + (____) + (____) (5) WT. OF WATER DISPLACED = $[(A)^2 (\pi /4) (E) + (C)^2 (\pi /4) (D)]$ (62.4) C. DESIGN MINIMUM FLOW: $= [(\underline{\hspace{1cm}})^2 (\pi /4) (\underline{\hspace{1cm}}) + (\underline{\hspace{1cm}})^2 (\pi /4) (\underline{\hspace{1cm}})] (62.4) =$ $\frac{ADF \times 0.20}{1440} = \frac{() \times 0.20}{1440}$ $\frac{\text{TOTAL WEIGHT}}{\text{WT. OF WATER DISPLACED}} = \frac{(4)}{(5)} = \frac{() \text{ lb.} \downarrow}{() \text{ lb.} \downarrow} = \frac{() \text{ lb.} \downarrow}{() \text{ lb.} \downarrow}$ SAFETY FACTOR = D. DESIGN PUMP CAPACITY (MINIMUM REQUIRED) E. VELOCITY IN FORCE MAIN AT MAX. PUMPING RATE = _____ FEET/SECOND III. WETWELL DESIGN (DUPLEX SYSTEM) A. DESIGN CRITERIA: 1. MAXIMUM PUMP MOTOR CYCLE RATE = 6 STARTS PER HOUR 2. MAXIMUM DETENTION TIME AT MINIMUM FLOW = 30 MINUTES B. PUMP CONTROL LEVEL SETTINGS: 1. PUMP CYCLING RATES ARE AT A MAXIMUM WHEN INFLOW EQUALS ONE-HALF THE DESIGN PUMPING RATE OF (II.B.) G.P.M. 2. WETWELL VOLUME REQUIRED BETWEEN LEAD PUMP START AND PUMP V = CYCLE PERIOD x (1/2) PUMP RATE $V = \frac{10 \text{ MIN.} \times (1/2) () \text{ G.P.M.}}{2} = \frac{\text{GALLONS}}{2}$ 3. WETWELL DIAMETER (D) = _____ FEET WETWELL VOLUME = $\frac{\pi(D)^2 \times 7.48 \text{ GAL./C.F.}}{4}$ = $\frac{7.48(\pi)(D)^2}{4}$ 4. WETWELL LEVEL CHANGE BETWEEN PUMP STOP AND LEAD PUMP START = (III.B.2.-GAL.) / (III.B.3. - GAL./FT.DEPTH) = FEET DESIGN FOR: _____INCHES 5. CONTROL ELEVATIONS: TOP OF SLAB INFLUENT INVERT HIGH WATER ALARM ELEV. _____ LAG PUMP ON LEAD PUMP ON ALL PUMPS OFF ELEV. _____ IV. SYSTEM CURVE CALCULATIONS A. FRICTION LOSS: 1. PUMPING STATION PIPING FRICTION LOSS (ea.) a. TEE b. 90° ELBOW c. CHECK VALVE d. GATE VALVE e. SIDE OUTLET CROSS f. WYE g. OTHER: (SIZE) "% (TYPE) PIPE LENGTH = TOTAL EQUIVALENT LENGTH = 2. FORCE MAIN PIPING: FRICTION LOSS (ea.) (INCHES) a. TEE b. 90° ELBOW c. CHECK VALVE d. GATE VALVE e. SIDE OUTLET CROSS g. OTHER: _______PIPE LENGTH = ______ TOTAL ÉQUIVALENT LENGTH = _____ B. STATIC HEAD: = ELEV. _____ FT. = ELEV. ____ FT. = ____ FT. 1. PIPE CENTER LINE AT DISCHARGE POINT 2. LOW WATER LEVEL (ALL PUMPS OFF) 3. TOTAL STATIC HEAD (B.1.- B.2.) C. PRESSURES AT POINT OF CONNECTION: BEST & WORST CASE SYSTEM CURVES PROVIDED BY WATER RESOURCE SERVIES, INFRASTRUCTURE

FLOTATION CALCULATIONS - CONCRETE WET WELL



SYSTEM vs. PUMP PERFORMANCE CURVE

PUMP MAN	IUFACTURER:	PUMP MODEL:		RPM:	HP:
GPM:	TDH:	IMPELLER DIA./NO.:	PHASE:	VOLTS:	AMPS:



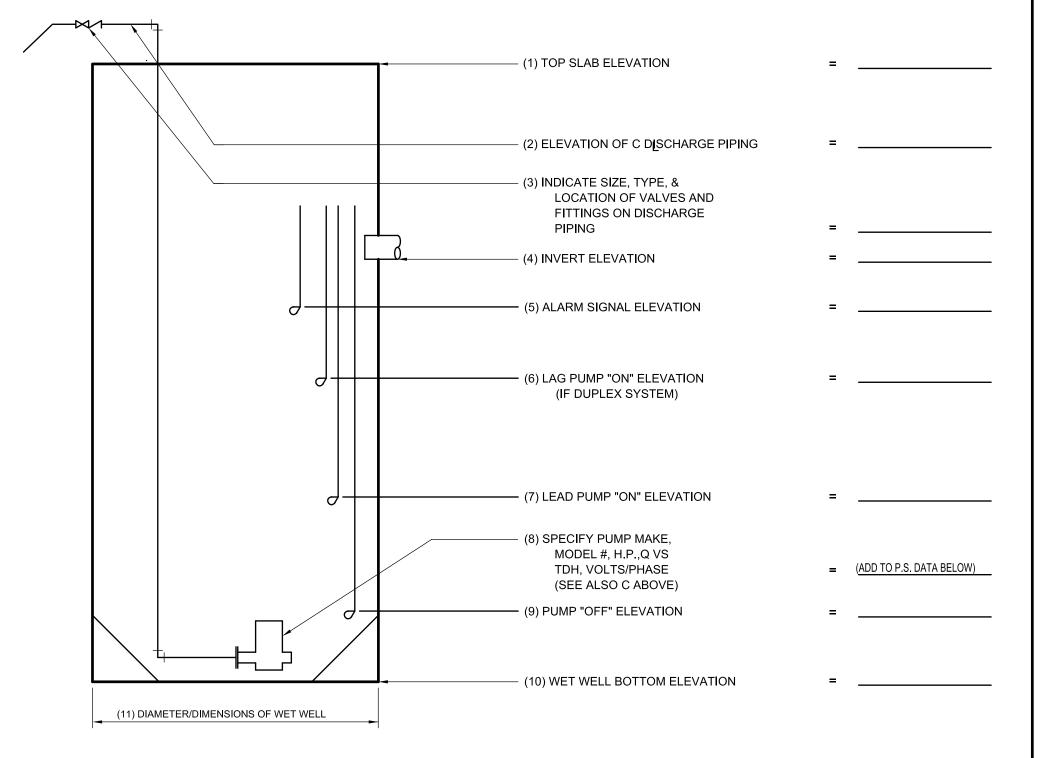
SYSTEM HEAD COMPUTATION - TOTAL LOSSES IN FEET

										FLC)W RA	TE (G	.P.M.)								
ITEM	0	2 5	5 0	7 5	100	1 2 5	150	1 7 5	200	2 2 5	2 5 0	275	3 0 0	3 2 5	3 5 0	3 7 5	4 0 0	4 2 5	4 5 0	4 7 5	500
A. FRICTION LOSS:																					
1. EQUIVALENT LENGTH PUMPING STATION PIPING:L.F. OF"(TYPE) PIPE																					
2. EQUIVALENT LENGTH FORCE MAIN PIPING:L.F. OF																					
B. STATIC HEAD: FEET																					
C. DESIGN PRESSURE AT POINT OF CONNECTION (BEST/WORST SYSTEM CURVES)																					
D. TOTAL LOSS (FEET): (BEST/WORST CASE)																					

PLANS FOR PRIVATELY-OWNED PUMPING STATIONS MUST PROVIDE

THE FOLLOWING INFORMATION AS A MINIMUM REQUIREMENT

- A) SITE PLAN SHOWING PUMPING STATION LOCATION AND POINT OF CONNECTION.
- B) PLAN AND PROFILE OF PUMPING STATION.
- C) SYSTEM vs. PUMP PERFORMANCE CURVE (PLOT ON GRAPH PROVIDED)
- D) COPY OF MANUFACTURES PUMP PERFORMANCE CURVES.
- E) PROVIDE DATA FOR ITEMS (1) THROUGH (11) AS SHOWN BELOW.
- F) PUMP STATION DATA.



PUMP STATION DATA (FOR PRIVATELY-OWNED STATIONS)								
DEVELOPMENT TO BE	SERVED:		LOCATION (NEAREST CROSS STREET):					
SECTION:	TOWNSHIP:	RANGE:	POWER CO. POLE/PAD NO.:					
	T POINT OF CONNECTION I. x 2.31 = FEET	ON:	AVERAGE DAILY FLOW (GPD): PEAK FLOW (GPM):					
WETWELL DIAMETER	(FEET):	WETWELL VOLUME (G	ALS./FT. DEPTH):	WETWELL DEPTH	H (FEET):			

NOTES:

SYSTEM HEAD VERSUS PUMP PERFORMANCE CURVES ARE TO BE SHOWN TO DETERMINE THE SYSTEM PERFORMANCE CAPABILITY AT THE FOLLOWING CONDITIONS:

A. CONVENTIONAL PUMPING STATION - FORCE MAIN (NON-MANIFOLD)

- 1. ONE PUMP RUNNING, IF DUPLEX STATION
- 2. ONE PUMP AND TWO PUMPS RUNNING, IF TRIPLEX STATION, ETC.
- 3. IF FORCE MAIN PROFILE RESULTS IN SIPHON, CURVES SHALL SHOW OPERATION AT START-UP (TO HIGH POINT OF PIPING) AS WELL AS FULL FLOW CONDITIONS

B. MANIFOLDED PUMPING STATIONS

- ALL CONDITIONS OUTLINED UNDER (A) ABOVE, AND THE FOLLOWING ADDITIONAL CONDITIONS
- 1. SIMULTANEOUS OPERATION OF ALL PUMPING STATIONS ON SYSTEM (WORST CASE)
- 2. OPERATION WHILE ALL REMAINING STATIONS ARE OFF (BEST CASE)

C. VARIABLE SPEED PUMPING STATIONS

- ALL APPLICABLE CONDITIONS UNDER (A) AND (B) ABOVE AND IN ADDITION:
- 1. OPERATING POINT, INCLUDING SPEED, AT PEAK, AVERAGE, AND MINIMUM FLOWS

SCALE			REVISIONS	
NONE				
	No.	DATE	DESCRIPTION	APPV'D.



HILLSBOROUGH COUNTY WATER RESOURCE SERVICES 925 E. TWIGGS STREET / TAMPA, FLORIDA 33602

PROJECT No.:	
FILE No.:	
DESIGNED BY:	
DRAWN BY:	
CHECKED BY:	
DATE:	OCT. 2015
SCALE:	NONE

NOT VALID UNLESS EMBOSSED SHEET PUMP STATION DESIGN PARAMETERS WITH ENGINEER'S SEAL I hereby certify that the work contained herein was prepared under my direct supervision and complies with the requirements of Chapter 471, Florida Statutes and Chapter 61G15, F.A.C. Signature: ____ Print Name: Florida Professional Engineer's Registration Number: